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Effect of Adding Mixture of (Concrete Waste and Asphalt Waste) on the Properties of Gypseous Soil

ABSTRACT

The Gypseous soil which is used is taken from Tikrit city in Salah Aldeen governorate and specially from Tikrit University from a depth (1.5-2) m. It's type is sandy gravely with a small percentage of silt and clay while the percentage of gypsum is (34-36) %. The tests on soil are standard proctor compaction, direct shear, collapsibility and California bearing ratio. The number of samples is (70). The effect of adding concrete waste (2, 4, 6 and 8%) and waste of Asphalt mixture (2, 4, 6 and 8) % on dry soil, as well as the effect of mixture of the optimum percentages of both additions on the properties of gypseous soil, is the aim of the study. Adding concrete waste at optimum percentage (6) % causes an increase in maximum dry density at (16)% and a decrease in optimum moisture content at (5) %. Whereas adding concrete mixture waste in its optimum percentage (2) % caused a decrease in maximum dry density values with an increase of optimum moisture content, and when adding a mixture of optimum percentage of waste, an increase happened in the M.D.D.(14)% with a decrease in O.M.C. at (4) %. Adding the optimum percentage (8) % for both, the value of cohesion increases (100) % when adding concrete waste with an increase in the angle of internal friction (14)% and a decrease in collapsibility in a percentage of (90)% , while adding waste of asphalt mixture shows an increase in cohesion value (112) % with a decrease in the angle of internal friction (2) % and a decrease in collapsibility in a percentage of (90) % , when adding mixture of optimum percentages the value of cohesion increase (108) % with an increase in angle of internal friction (14)% and a decrease in collapsibility in a percentage of (91) % . Values of California Bearing Ratio in dry and soaked condition increases (49) % when adding (8) % of concrete wastes which is the optimum percentage, while adding waste of asphalt mixture causes a decrease in the value of C.B.R. and the optimum percentage is (2) % , but adding optimum percentages mixture of them causes an increase in values of (52) % in dry condition and (53)% in soaked condition.

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تأثير إضافة مزيج (مخلفات الخرسانة ومخلفات الاسفلت) على خصائص التربة الجبسية

الخلاصة

التربة الجبسية المدروسة مأخوذة من مدينة تكريت في محافظة صلاح الدين وتحديداً من جامعة تكريت ومن عمق يتراوح (1.5-2) م، وإن نوعها هو رملية حصوية مع نسبة ضئيلة من الغرين والطين حيث إن نسبة الجبس لها (34-36)%. يهدف البحث إلى دراسة تأثير إضافة مخلفات الخرسانة بنسب (2، 4، 6 و 8) %، مخلفات الخلطة الاسفلتية بنسب (2، 4، 6 و 8) % على التربة الجافة ومزيجها الذي يمثل النسب التي أعطت أفضل نتائج للفحوصات وضمن النسب المدروسة. إن إضافة مخلفات الخرسانة بنسبة (6) % عملت على زيادة قيم الكثافة الجافة العظمى بمقدار (15) % مع نقصان في قيم محتوى الرطوبة الأمثل بمقدار (6.9) % أما إضافة مخلفات الخلطة الاسفلتية فقد أدت إلى نقصان في قيم الكثافة الجافة العظمى مع زيادة في محتوى الرطوبة الأمثل. إضافة مخلفات الخرسانة أدت إلى حصول زيادة في قيم التماسك بمقدار (100) % مع حصول زيادة في

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قيم زاوية الاحتكاك الداخلي بمقدار (13.5) %، وعند اضافة مخلفات الخلطة الاسفلتية فقد نتج عنها زيادة في قيم التماسك بمقدار (112) % مع حصول نقصان في قيم زاوية الاحتكاك الداخلي بمقدار (1.38) %، في فحص الانهيارية فان القيم تتناقص بزيادة نسب مخلفات الخرسانة ومخلفات الخلطة الاسفلتية. إن قيم نسبة التحمل الكاليفورني في الفحص الجاف ازدادت بمقدار (96.5) % عند اضافة مخلفات الخرسانة، أما عند اضافة مخلفات الخلطة الاسفلتية فقد تناقصت بمقدار (13.6) %، وفي فحص النماذج المغمورة فقد ازدادت بمقدار (96.4) % عند اضافة مخلفات الخرسانة، أما عند اضافة مخلفات الخلطة الاسفلتية فقد تناقصت بمقدار (40) %.

1. INTRODUCTION

Gypseous soil is found in many regions all over the world specially in the dry and semi dry regions, one of the regions is Iraq. It forms approximately (28.6%) from soils of Iraq and covers more than (12.5) Million hectors from the total area [1].

This type of soil exists in Iraq's basin and correlated with areas of Tigris and Euphrates rivers. It is distinguished in having a high percentage of gypsum reached to (80)% or more with thickness of five Meters and in some times It may cause a rocky solid gypsum horizon (petrogypsic) [2]. The waste of concrete and asphaltic mixture can be used in a good way to improve the properties of gypseous soil that suffering from problems, and that is so important from the economic side which decrease the cost of engineering projects. The gypseous soil is a stiff soil in dry condition but it loses its stiffness in the existence of water which is caused big crumbles because of the breaking of boundaries between the particles of soil due to the dissolve of gypsum. The researchers improve properties of soil in a specified ways, [3] studying the improvement of gypseous soil by pre-wetting and the result shows a large decrease in percentage of (foundation settlement / foundation width) specially in the third recycle of soaking with water and the value of this reduction is (91)%, [4] the effect of using stone columns on properties on gypseous soil is taken from Al-Door district in Salah Al-deen government with gypsum content (66)%, the results shows a decrease in the value of collapsibility in a percentage of (56)% and (53)% when the value of applied pressure is (32) kPa and (4408) kPa respectively. Abdullah [5] studied the effect of washing on the properties of gypseous soil in Samaraa city in Iraq, the conclusion is a decrease in dry density because of removing some of gypsum content after washing while the values of specific gravity are increased and decreased in the porosity of soil. Ibrahim and Schanz [6] studied the improvement of gypseous soil using Silica oil, the results show that this material improves the compressibility and shear strength of soil.

2. MATERIAL

2.1. Soil

Gypseous soil is taken from Tikrit city in Salah Al-Din government and specially from Tikrit University, at a depth (1.5-2 m) after removing the upper stratum of soil. Samples that taken are put in a plastic bags and transported to the laboratory. From Table 1 it can be concluded that the used soil is sandy gravelly soil with small percentage of silt and clay. Table 2 shows the results of physical and chemical tests for normal soil.

2.2. Concrete wastes and Asphalt mixture waste

Concrete waste is taken from Tikrit city which is founded in many places. This waste is crushed using a special crusher machine and then sieved using sieve No.

(40) to using it as an additive to gypseous soil in the dry condition.

Asphalt mixture waste is taken from Tikrit city which is founded in many places. This waste is crushed using a special crusher machine after freezing it for several hours to avoid the changing of asphalt to dough and sieving it using sieve No. (40) to using it as an additive to the soil.

Table 1

Results of sieve analysis test.

No. of sieve (ASTM)	Dia. of particle (mm)	(%) of Passing
3/8 in.	9.51	100
4	4.75	95.32
10	2.00	84.54
20	0.85	78.54
30	0.60	60.01
40	0.425	50.06
60	0.25	24.66
100	0.15	5.75
200	0.075	2.96

$C_u = 3.243$ and $C_c = 0.757$

Table 2

Results of physical and chemical tests.

No	Test	Results
1	Water Content (%)	8.5
2	Specific Gravity	2.59
3	Atterberg Limit	L.L. 26.5 P.L. NP
4	Gypsum Content (%)	34.27
5	pH value	8.06
6	Organic Matters (%)	0.51
7	Total Soluble Salts (%)	45.5
8	Solphate Content (%)	28
9	Type of Soil	SP
	Standard	M.D.D.
10	Proctor Compaction	(kN/m ³) 15.3 OMC(%) 14.14
		c (kPa) 37.5
11	Direct Shear	□° 39.64
12	Potential Collapse (%)	12.061
		Dry condition 23.4
13	CBR	Soaked condition 2.8

2.3. Method of Mixing

The soil is sieved by using specific sieve for each test and heated in the oven, the degree is (40°-50°) to

prevent drying the gypsum [7] and then it be taken a specific weight of soil depending on the type of the test, adding weight percentage of the weight of dry soil and mixed by hand or plastic bag with continuous movement until It becomes a homogenous mixture.

3. RESULTS AND DISCUSSION
3.1. Standard Procter Test

Figs. 1 and 2 show the relationship between the effect of percentages adding on maximum dry density and optimum moisture content, Table 3 shows the result of this test.

It can be concluded that optimum percentage of concrete wastes is (6%) which has the maximum value of dry density and with value of optimum moisture content. The optimum percentage of asphalt mixture wastes is (2%) which has the maximum value of dry density and with value of optimum moisture content, the standard of this test is (ASTM D698-12)

The reason behind the increase of max. dry density until the percentage (6) % belong to adding materials that passed from sieve No. (40) which improves the gradation of used soil so It leads to the increase [8]. While the reason of decrease in dry density at (8%) is that the adding is filling materials cohesionless caused a decreased in cohesive soil materials [9].

The asphalt waste mixture causes a decrease in the dry density and an increase in moisture content, the main reason for decreasing is the density of added mixture is less than density of soil while the reason of increasing of moisture content is the viscosity of added material which is more than water's viscosity so that leads to an increase [10].

Table 3
 Results of proctor compaction test.

No.	Type of Addition	Addition (%)	M.D.D. (kN/m ³)	OMC (%)
1	No additive	0	15.38	14.14
2		2	15.88	14.00
3	Concrete	4	16.55	13.83
4	Wastes	6	17.70	13.50
5		8	17.30	13.22
6	Asphaltic	2	15.25	14.28
7	mixture	4	15.04	14.58
8	Wastes	6	14.80	14.76
9		8	17.76	13.64
10	Opt. (%) of (Concrete+ Asphaltic) wastes	6+2	17.55	13.64

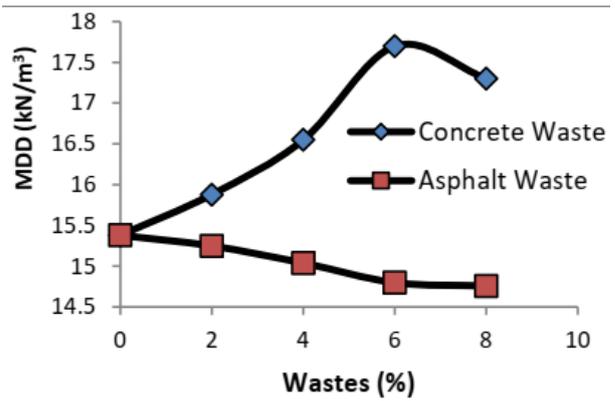


Fig. 1. Relationship between percentages of wastes and maximum dry density

3.2. Direct Shear Test

Figs. 3 and 4 show the relationship between normal stress and shear stress for four cases (natural samples, curing samples with concrete waste, curing samples by adding asphalt mixture and with the optimum percentage of them), The results of direct shear test are showed in Table 4.

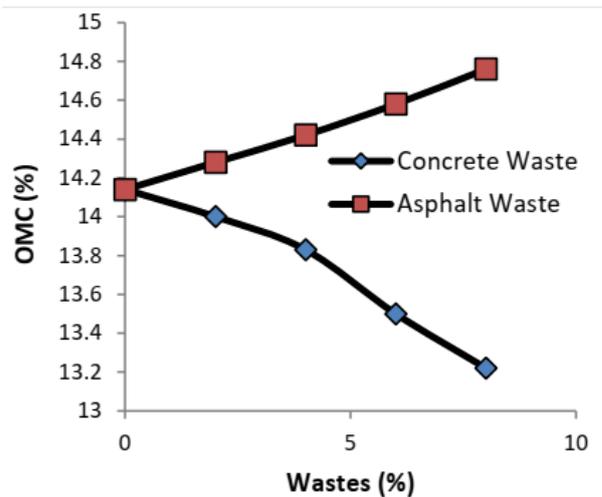


Fig. 2. Relationship between percentages of wastes and optimum moisture content

Using equation of Terzaghi and Meyerhof by substituting the values of direct shear test it can be concluded that the optimum percentages of adding were (8)% which the maximum value of bearing capacity for soil's samples, and in the fourth case the optimum percentages are used to have the maximum value of bearing capacity, The standard of this test was (ASTM D3080-11).

Adding concrete mixture to the soil caused an increase in strength of soil, the reason behind that is, these materials free the calcium which created (Attringite), while the angle of friction is increased with the increase of adding mixture, and the reason was that material caused an increase in the friction between soil particles [11,12].

Adding concrete waste mixture to the soil made an increase in the cohesion between soil particles and reduce

the permeability of water through it and the reason behind that is only physical factors without any chemicals interactions happened, [10].

Many researches find that strength of soils increases with the increase of asphaltic materials but the percentage of adding should not exceed the optimum percentage otherwise that will lead to decrease in strength of soil [13]. While the angle of friction decreased with the increase of asphaltic material and that happened because these materials facilitate the slipping of particles.

Table 4

Results of direct shear test.

No.	Type of Addition	Addition (%)	c (kPa)	ϕ°
1	No additive	0	37.5	39.64
2	Concrete Wastes	2	50	72.93
3		4	61.66	43.00
4		6	74.50	44.63
5		8	75.00	45.00
6		2	55.00	39.92
7	Asphaltic	4	65.00	39.50
8	mixture Wastes	6	70.50	39.24
9		8	79.50	39.1
10	Opt. (%) of (Concrete+Asphalti) wastes	8+8	78.00	45.00

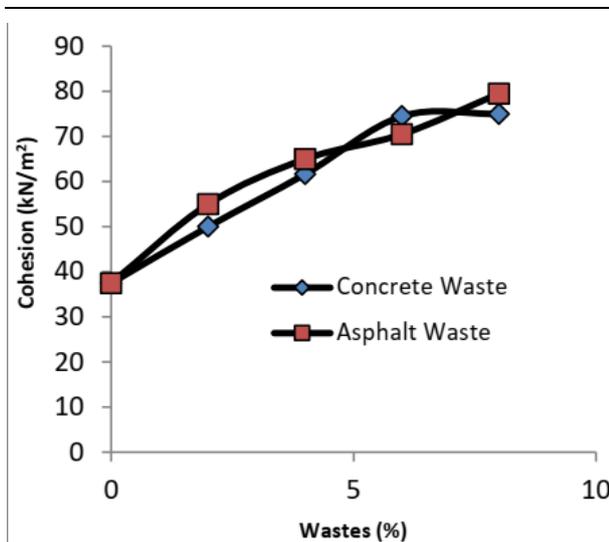


Fig. 3. Relationship between percentages of wastes and cohesion.

3.3. Collapse Potential Test

Fig. 5 shows the relation between percentages of waste with the values of collapse potential. The optimum percentages of adding (concrete waste and asphalt mixture waste) was (8%) because it has the minimum value of potential collapse. In the fourth stage from work steps the optimum percentages of each adding are used to

reach the best value of potential collapse, the values of potential collapse and initial void ratio for the samples of gypsum soils are shown in Table 5. From the show results It can be concluded that the potential collapse decrease with the increase of adding mixtures, the reason behind this decrease belongs to many causes such as this adding acts as a cover for the gypseous particles subsequently reduce or prevent the effect of water on the particles, and the concrete waste mixture act as a bonding materials between soil particles [14].

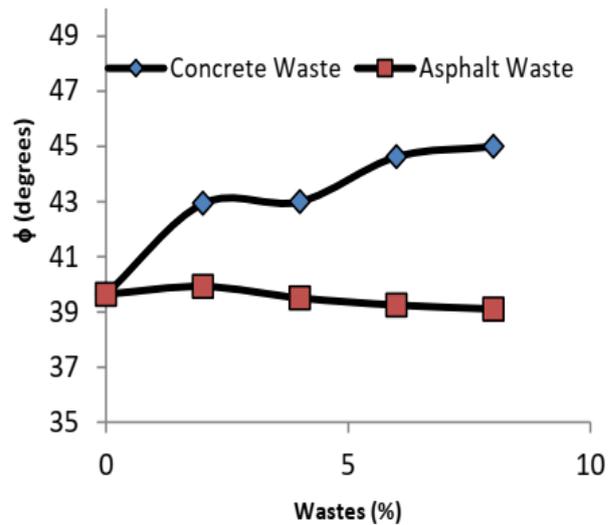


Fig. 4. Relationship between percentages of wastes and angle of internal friction.

3.4. California Bearing Ratio Test

Figs. 6-9 show the relation between No. of blows and C.B.R. for the waste for both conditions. The results of this test for both conditions are shown in Table 6 for natural soil and curing soil with concrete and asphaltic mixtures wastes using optimum moisture content. The results for both condition show an increase in C.B.R. values when the concrete waste increase and the optimum percentage is (8%), while the adding of asphaltic mixture causes a decreased in the values and the optimum percentage is (2%), The standard of this test is (ASTM D1883-99).

The reason of the increase of the values for C.B.R. in both conditions (dry and soaked) is the concrete waste mixture caused an increase in the density of the samples where these materials a filling materials cause an increased in the friction between soil particles which lead to increase soil strength at all [15]. While the reason of decrease in values of C.B.R. in both conditions when the asphalt waste mixture added, the reason is as a result for the decrease in density as mentioned in above.

Table 5
Results of Collapse test.

No.	Type of Addition	Addition (%)	Initial void ratio (e_0)	Collapse potential (C_p)
1	No additive	0	0.7724	7.7804
2	Concrete	2	0.7633	1.0151
3	Wastes	4	0.6401	1.0060
4		6	0.6411	0.9444
5		8	0.5302	0.7384
6	Asphaltic	2	0.7505	1.0854
7	mixture	4	0.6355	0.9293
8	Wastes	6	0.6543	0.8704
9		8	0.5278	0.7068
10	Opt. (%) of (Concrete+ Asphaltic) wastes	8+8	0.5322	0.6433

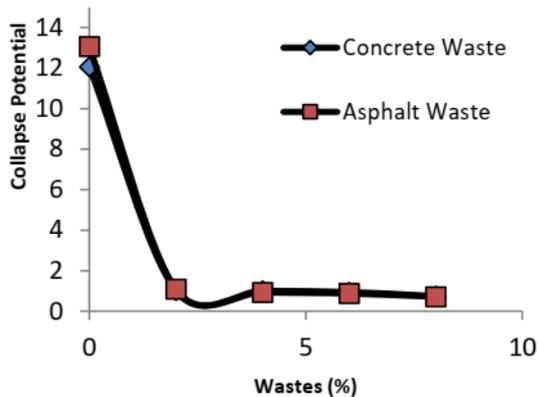


Fig. 5. Relationship between percentages of wastes and collapse potential.

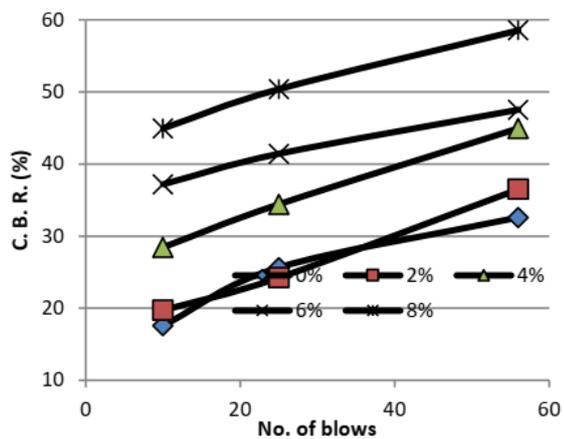


Fig. 6. Relationship between No. of blows with C.B.R. for concrete wastes (dry condition)

Table 6
Results of California bearing ratio test.

No.	Type of Addition	Addition (%)	CBR	
			Dry Cond.	Soaked Cond.
1	No additive	0	23.4	2.8
2	Concrete	2	21.6	2.9
3	Wastes	4	30.5	3.7
4		6	38.5	4.5
5		8	46	5.5
6	Asphaltic	2	22.8	2.6
7	mixture	4	21.4	2.6
8	Wastes	6	21.3	2.2
9		8	20.6	2.0
10	Opt. (%) of (Concrete+ Asphaltic) wastes	8+2	49	6.0

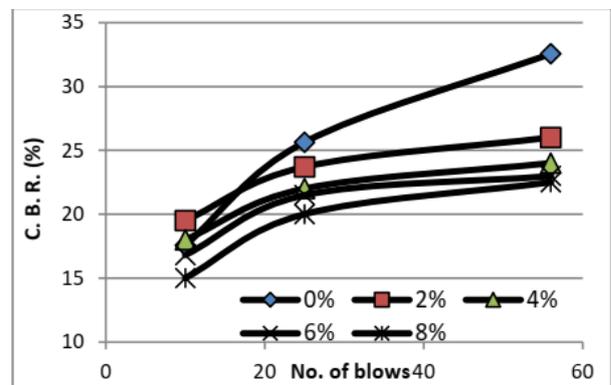


Fig. 7. Relationship between No. of blows with C.B.R. for Asphalt wastes (dry condition).

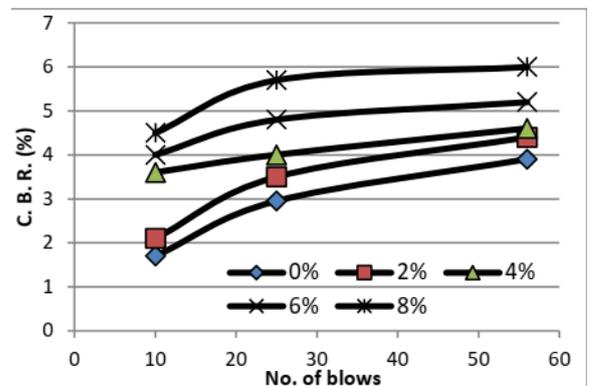


Fig. 8. Relationship between No. of blows with C.B.R. for concrete wastes (soaked condition).

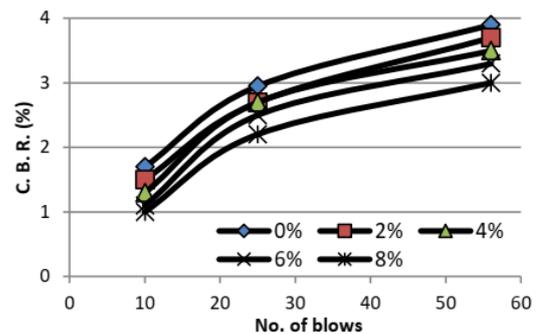


Fig. 9. Relationship between No. of blows with C.B.R. for Asphalt wastes (soaked condition)

4. CONCLUSION

1. The values of M.D.D. in standard Procter compaction test increase with the increase of concrete wastes, in which 6% represent the optimum value, while the values of M.D.D. decrease with the increase of asphalt mixture wastes and the optimum value is 2%, while adding of optimum percentages of wastes, the value of maximum dry density increase in a magnitude of 14%. The values of optimum moisture content decrease in a magnitude of 5% with the increase of concert wastes and the optimum value is 8%, the adding of asphalt mixture caused an increase in values of optimum moisture content in 2% which represents the optimum value. In the fourth stage of work steps the values of optimum moisture content are decreased in a magnitude of 4%.
2. The values of cohesion and angle of internal friction increase with the increase of concrete wastes, in which the cohesion increases in a magnitude of 100% and angle of internal friction in a magnitude of 14%, the value 8% represent the optimum percentages. When asphalt mixture wastes are added, the cohesion increase in a magnitude of 112% with a decrease in the angle of internal friction in a magnitude of 2% and the optimum percentage is 108% with an increase in the angle of internal friction in a magnitude of 14%.
3. The values of potential collapse in collapse test are decreased in a magnitude of 90% when the waste is added separately and the optimum percentage is 8%. While the value of potential collapse decreases in a magnitude of 91% when the waste is added composedly.
4. The value of C.B.R. in dry and soaked conditions increases in a magnitude of 49% when the percentage 8% of concrete waste is added, while the adding of asphalt mixture caused a decrease in vale of C.B.R. and the optimum percentage is 2%. When both of waste are added the values increase in a magnitude of 52% in dry condition and 53% in soaked condition.

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